Zone A Workshop

How to determine Base Flood Elevation (BFE) (100-year flood)



New Hampshire Office of Energy and Planning

U.S. Geological Survey Water Science Center New Hampshire - Vermont

Zone A Workshop Outline

- I. Sources of Flood and Watershed Information
- II. How to Determine BFE: Simple Methods Detailed Methods
- **III.** Example of Determining Zone A BFE
- **IV.** Questions and Answers

Sources of Flood and Watershed Information Previous Flood Studies

- Draft flood studies (new or re-study) FEMA
- Flood control projects USACE, NRCS, Dam Bureau (NHDES)
- High flow design analysis (e.g. bridges)
 FHA, NHDOT, County Highways, Public Works

Sources of Flood and Watershed Information

Federal Agencies

- FEMA Federal Emergency Management Agency
- USACE U.S. Army Corps of Engineers
- FHA Federal Highway Administration
- USGS U.S. Geological Survey
- NRCS Natural Resources Conservation Service
- NOAA National Oceanic and Atmospheric Administration

Sources of Flood and Watershed Information

State / Regional Agencies

- NHOEP N.H. Office of Energy and Planning
- NHDOT N.H. Department of Transportation
- NHDES N.H. Department of Environmental Services
- **RPCs** Regional Planning Commissions
- NERCC Northeast Regional Climate Center

Sources of Flood and Watershed Information

Local Agencies County Highway Department

City / Town Engineer

Department of Public Works

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Extrapolation upstream from existing study







Contour Interpolation using topographic and Zone A maps





River Zone A

Lake/Pond Zone A











Contour Interpolation



Contour Interpolation



Contour interval = 40ft

Left Bank: Zone A Boundary = 843 ft

Right Bank Zone A Boundary = 837 ft

843-837 = 6 ft OK (6<40/2)

BFE = 837+40/2 = <u>857 ft</u>

Contour Interpolation using topographic and Zone A maps









Contour Interpolation



Contour interval = 40ft

Lowest perimeter point: Zone A Boundary = 1010 ft

Highest perimeter point: Zone A Boundary = 1021 ft

1021-1010 = 11 ft OK (11<40/2)

BFE = 1010+40/2 = <u>1030 ft</u>

Zone A Workshop Outline

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- **III.** Example of Determining Zone A BFE
- **IV.** Questions and Answers

Steps to Determine BFE

1. Hydrology: 100-year discharge (flow, ft³/s)

2. Survey: river and structures

3. Hydraulics: compute water elevation (BFE)





Gaged or Ungaged





Three common approaches:

- Discharge / Drainage area
- Generalized equations (USGS Streamstats)
- Computer models

Discharge / Drainage Area



Generalized Equations

Rational Formula
Q = C * i * A

> Regression Equation Q = $153A^{0.865}L^{-0.336}E^{0.125}Y^{-0.420}$

Q = discharge, C = coefficient, i = rainfall intensity, A = drainage area L = % lakes/ponds, E = % elevation >1200ft, Y = latitude factor









Basin Characteristics Report

Date: Fri Sep 12 2008 09:03:29 Latitude (NAD83): 43.9886 (43 59 19) Longitude (NAD83): -72.1495 (-72 08 58)

Parameter ///////////////////////////////////	<u>Value</u>
Area in square miles	145
Mean annual precipitation in inches	40.4
Y coordinate of the centroid in map coordinates	174949.7
Percent of area covered by lakes and ponds	0.15
High Elevation Index - Percent of area with elevation > 1200 ft	67.1



Streamflow Statistics Report

Site Location: Vermont Latitude: 43.9886 Longitude: -72.1495 Drainage Area: 145 mi2

Streamflow Statistics						
Statistic	Flow (ft ³ /s)	Prediction Frror	Equival ent	90-Percent Prediction Interval		
		(percent)	years of record	Minimum	Maximum	
Q2	4000	42	1.4	2080	7680	
Q5	5810	40	2.3	3070	11000	
Q10	7150	41	3.2	3740	13700	
Q25	8990	42	4.6	4680	17300	
Q50	10400	43	5.5	5360	20300	
Q100	11900	44	6.3	5990	23800	
Q500	15700	49	7.6	7360	33600	



Hydrology: 100-yr flow at USGS gaging station

			20 Percent Duration	1750	a bit bit and a second			Survey Street
and the second			and rendering or an advert	1,230	CUDIC rees per second	November_Mean_Flow	922.000	cubic feet per second
			25_Percent_Duration	996	cubic feet per second	November_STD	628.000	cubic feet per second
Streamflow Statistics			30_Percent_Duration	824	cubic feet per second	October_Mean_Flow	541.000	cubic feet per second
			40_Percent_Duration	595	cubic feet per second	October_STD	408.000	cubic feet per second
Statistic Name	Value	Units	5_Percent_Duration	3340	cubic feet per second	September_Mean_Flow	393.000	cubic feet per second
			50_Percent_Duration	458	cubic feet per second	September_STD	345.000	cubic feet per second
Peak-Flow Statistics			60_Percent_Duration	365	cubic feet per second			
10 Year Peak Flood	30400.0	cubic feel per second	70_Percent_Duration	297	cubic feet per second	General Flow Statistics		
100 Year Peak Flood	57600.0	cubic feel per second	75_Percent_Duration	270	cubic feet per second	Average_daity_streamflow	937.562	cubic feet per second
2 Year Peak Flood	15200.0	cubic feet per second	80_Percent_Duration	242	cubic feet per second	Maximum_daily_flow	33900	cubic feet per second
200 Year Peak Flood	67600.0	rathir feet per second	90 Percent Duration	183	cubic feet per second	Minimum daily flow	66	cubic feet per second
25 Year Dask Floor	40300.0	minis fast per second	95 Percent Duration	150	cubic feet per second	Std Dev of daily flows	1408.912	cubic feet per second
5 Year Dask Dovel	23600.0	which and per second	99 Percent Duration	107	cubic feet per second			
50 Year Peak Floor	49500.0	cubic feet per second				Base Flow Statistics		
500 Year Bask Engel	92200.0	which and par second	Annual Flow Statistics			Average BEL value	0.537	dimensionless
Los Maan of Assure Dealer	4 1000	Log have 10	Daily flow years	55 000	VERY .	Number of years to compute RFI	80	Vears
Log Skew of Annual Peaks	0.0000	Log base 10	Mean Annual Flow	921.000	cubic feet per second	Std dev of annual BFI values	0.065	dimensionless
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WHC_SKEW	0.2140	Log base 10	August Maan Flow	312,000	cubic fast per second			
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Physical State Street Str			December Mean Flow	750.000	cubic feet per second			
Flood-Volume Statistics		and the second second second	December 970	550,000	cubic fast per second			
7_Day_10_Year_Maximum	8650.00	cubic feet per second	February Mean Flow	463,000	cubic faet per second			
7_Day_2_Year_Maximum	5400.00	cubic teet per second	Echanter STD	347 000	white fast per second			
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7_Day_2_Year_Low_Flow	138.000	cubic feet per second	burg_STD	200.000	court foot per second			
7_Day_20_Year_Low_How	88.300	cubic feet per second	June_Mean_Flow	443,000	could real per second			
Low_flow_years	54,000	years	June_STD	442.000	cubic first per second			
1 Section Sect			March_Moah_Flow	868.000	cubic reet per second			
Flow-Duration Statistics	25.52		March_STD	901.000	cubic reet per second			
1_Percent_Duration	6880	cubic feet per second	May_Mean_Flow	2380.00	cubic relet per second			
10_Percent_Duration	2170	cubic feet per second	May_s rD	937.000	cubic nee per second			
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StreamStats Data-Collection Station Report

USGS Station Number 01064500 Station Name Saco River near Conway, NH

Descriptive Information

Station Type	Gaging Station, continuous record
Regulated?	Undefined
Period of Record	
Latitude (degrees NAD83)	43.99084167
Longitude (degrees NAD83)	-71.09047778
Hydrologic unit code	01060002
County	003-Carroll

Physical Characteristics

Characteristic Name	Value	Units
24_Hour_2_Year_Precipitation	3.3000	inches
Contributing_Drainage_Area	385	square miles
Drainage_Area	385.000	square miles
Main_Channel_Length	35.100	miles
March_Snow_Water_Equivalent	6.6700	inches
Mean_Annual_Precipitation	50.570	inches
Mean_Annual_Snowfall	101.000	inches
Mean_Basin_Elevation	1860.00	feet
Mean_Min_January_Temperature	9.0000	degrees F
Percent_Forest	94.800	percent
Percent_Lakes_and_Ponds	0.4200	percent
Percent_Storage	0.4800	percent
Soil_Infiltration	5.6000	inches
Stream_Slope_10_and_85_Method	50.990	feet per mi

Streamflow Statistics

Statistic Name	Value	Units
Peak-Flow Statistics		
TU_Year_Peak_Flood	30400.0	cubic feet per second
100_Year_Peak_Flood	57600.0	ubic feet per second
2_Year_Peak_Flood	15200.0	cubic feet per second
200_Year_Peak_Flood	67600.0	cubic feet per second
25_Year_Peak_Flood	40300.0	cubic feet per second
5_Year_Peak_Flood	23600.0	cubic feet per second
50_Year_Peak_Flood	48500.0	cubic feet per second
500_Year_Peak_Flood	83200.0	cubic feet per second
Log_Mean_of_Annual_Peaks	4.1800	Log base 10
Log_Skew_of_Annual_Peaks	0.0020	Log base 10
Log_STD_of_Annual_Peaks	0.2320	Log base 10
Mean_Annual_Flood	9850.00	cubic feet per second
Systematic_peak_years	56.000	years
WRC_Mean	4.1910	Log base 10
WRC_Skew	0.2140	Log base 10
WRC_STD	0.2323	Log base 10
Flood-Volume Statistics		
7_Day_10_Year_Maximum	8650.00	cubic feet per second
7_Day_2_Year_Maximum	5400.00	cubic feet per second
7_Day_50_Year_Maximum	12000.0	cubic feet per second
Low-Flow Statistics		
7_Day_10_Year_Low_Flow	96.700	cubic feet per second
7_Day_2_Year_Low_Flow	138.000	cubic feet per second
7_Day_20_Year_Low_Flow	88.300	cubic feet per second
Low_flow_years	54.000	years
Flow-Duration Statistics		
1_Percent_Duration	6880	cubic feet per second
10_Percent_Duration	2170	cubic feet per second
USGS Gaging Stations in New Hampshire and Vermont



Watershed Models

- > NRCS: TR-55, TR-20
- Corps of Engineers: HEC-1

Input data needed include:

- Watershed characteristics (area, slope, land cover, soils)
- Channel conveyance (slope, shape, roughness)
- 100-yr rainfall intensity
- Flood storage
- Structures (dams, bridges)

<u>Output</u> data is:

Flood hydrograph (peak = 100-yr discharge)

Flood Hydrograph





Steps to Determine BFE

Hydrology: 100-year discharge (flow, ft³/s)
 Survey: river and structures

3. Hydraulics: compute water elevation (BFE)

Field Survey



- Vertical datum
- **River cross sections** Number, elevations & distances
- Roughness coefficient Manning's "n"

Structures

Dams, bridges, culverts

NGVD29 National Geodetic Vertical Datum of 1929

NAVD33 North American Vertical Datum of 1988

Tie all survey points to known Reference Mark (RM)





COMPARISON OF VERTICAL DATUM ELEMENTS

	<u>NGVD 29</u>	<u>NAVD 88</u>
DATUM DEFINITION	26 TIDE GAUGES	FATHER'S POINT/RIMOUSKI
	IN THE U.S. & CANADA	QUEBEC, CANADA
BENCH MARKS	100,000	450,000
LEVELING (Km)	102,724	1,001,500
GEOID FITTING	Distorted to Fit MSL Gauges	Best Continental Model



Vertical Datum on FEMA's County DFIRMs in NH

- National Geodetic Vertical Datum (NGVD) of 1929
 - Grafton Strafford
 - Rockingham
- North American Vertical Datum (NAVD) of 1988
 - Cheshire Hillsborough (prelim)
 - Sullivan

– Merrimack (prelim)

Field Survey



Vertical datum

River cross sections

Number, elevations & distances

 Roughness coefficient Manning's "n"

Structures

Dams, bridges, culverts

River Cross Sections

• Minimum 1 x-sec for small lot Uniform flow, no obstructions

Minimum 2 x-secs for large lots

< 500 ft between x-secs if $\triangle WSE > 1$ ft





River Cross Sections

- Represent channel changes Slope, shape, roughness
- Show discharge changes
 Tributary inflow



River Cross Section



River Cross Sections



Field Survey



- Vertical datum
- River cross sections Number, elevations & distances

Roughness coefficient

Manning's "n"

Structures

Dams, bridges, culverts

Field Survey: *n* values



• Take photos and notes

Lined channels	n [s/m ^{4/3}]	kst [m ^{1/3} /s]
Cement Plaster	0.011	90.91
Wood planed	0.012	63.33
Wood unplaned	0.013	76.92
Concrete, troweled	0.012	63.33
Concrete, wood forms	0.015	66.67
Rubble in cement	0.020	50.00
Asphalt smoth	0.013	76.92
Asphalt rough	0.016	62.50
Natural		
Channels	n	kst
Gravel beds, straight	0.025	40.00
Gravel beda		
plus large boulders	0.040	25.00
Earth, straigh		
with some grass	0.026	38.46
Earth winding		
no vegetation	0.030	33.33
Earth winding	0.050	20.00

Field Survey: *n* values



Field Survey

n = 0.026



No. 327 upstream from right bank below section 3, Indian Fork below Atwood Dam, near New Cumberland, Ohio. 16

n = 0.026





No. 366 upstream toward right bank from section 7, Beaver Kill at Cooks Falls, N.Y.

60

Field Survey

n = 0.043



No. 359 upstream along left bank from below reach, Esopus Creek at Coldbrook, N.Y.

n = 0.043

n = 0.052; 0.047

Field Survey

n = 0.052



No. 1179 upstream from section 2, South Beaverdam Creek near Dewy Rose, Ga.

145

Field Survey



- Vertical datum
- **River cross sections** Number, elevations & distances
- Roughness coefficient Manning's "n"

Structures

Dams, bridges, culverts

Field Survey: Bridges

- Cross sections Approach & Exit
- Bridge geometry

 Dimensions
 Roadway
 Wingwall
 Piers, Skew



Field Survey: **Bridges**



Field Survey: Culverts









Field Survey: Culverts

- Cross sections
 Exit (& approach)
- Culvert geometry

 H x W x L
 Material & Type
 Slope (elevations)

 Entrance shape

 wingwalls, mitered, rounding
 Roadway





Field Survey: Dams



- Sluice gates, Flashboards, Spillway, Turbines
- Flow Regulation
- Standard Operating Procedures

Steps to Determine BFE

1. Hydrology: 100-year discharge (flow, ft³/s)

2. Survey: river and structures

3.Hydraulics: compute water elevation (BFE)

Hydraulics: Base Flood Elevation

Normal & Critical Depths

• Step Backwater Quick2, HEC-RAS

Structures

Weir and Conduit Flow

Hydraulics: Normal Depth

- Uniform, Steady Flow
- No Obstructions
- Water Surface parallel to Bed Slope



Manning Equation: $Q = \frac{1.49 \text{ A } \text{R}^{\frac{2}{3}} \text{ S}^{\frac{1}{2}}}{n}$

Hydraulics: Critical Depth

- Minimum specific energy
- Deeper is <u>sub</u>-critical flow (slow)
- Shallower is <u>super-critical</u> flow (fast)



Hydraulics: Supercritical Flow



$BFE \ge Critical Depth$

 $H_v > \frac{1}{2} D_h$

Hydraulics: Step Backwater

- Steady, Non-Uniform Flow
- Water Surface not parallel to Bed Slope

 $d_1 \neq d_2$ $v_1 \neq v_2$



Hydraulics: Step Backwater

- Start with known depth downstream normal depth, weir flow, etc.
- Then work upstream step-by-step compute energy & depth at each cross section
- Based on energy losses between cross-sections f(distance, slope, roughness, etc.)


Hydraulics: Base Flood Elevation

Normal & Critical Depths

• Step Backwater Quick2, HEC-RAS

• <u>Structures</u> Weir and Conduit Flow

Hydraulics: Bridges



Hydraulics: Bridges Flow over roadway and/or bridge deck



$\mathbf{Q} = \mathbf{k} \mathbf{C} \mathbf{b} \mathbf{H}^{1.5}$

- K = submergence factor
- C = weir coefficient
- b = weir width
- H = water height above weir crest

Weir Flow

Hydraulics: Bridges Flow through opening



- Energy Losses from Contraction & Expansion
- Wingwall design, channel cross sections
- Computer model (e.g. HEC-RAS)

Hydraulics: **Bridges**







Tranquil flow throughout

$$Q = CA_3 \sqrt{2g(h_1 + \alpha_1(v_1^2/2g) - h_3 - h_{f_{1,2}} - h_{f_{2,3}})}$$

Hydraulics: Culverts



$$Q = CA_{c}\sqrt{2g(h_{1}+\alpha_{1}(v_{1}^{2}/2g)-d_{c}-h_{f_{1,2}}-h_{f_{2,3}})}$$

Hydraulics: Culverts



$$Q = CA_0 \sqrt{2g(h_1 - z)}$$

Hydraulics: Culverts



 $Q = CA_0 \sqrt{2g(h_1 - h_4)/(1 + 29C^2n^2L/R_0^{4/3})}$



 $\mathsf{Q} = \mathsf{C}\mathsf{A}_0 \sqrt{2g(h_1 - h_4)/(1 + 29\mathsf{C}^2 n^2 \mathsf{L}/\mathsf{R}_0^{4/3})} + k\mathsf{C}b\mathsf{H}^{3/2}$

Zone A Workshop Outline

- I. Sources of Flood and Watershed Information
- II. How to Determine BFE: Simple Methods Detailed Methods
- III. Example of Determining Zone A BFE

IV. Questions and Answers

Example: Normal Depth BFE



 $Q_{100} = 7,200 \text{ ft}^3/\text{s}$

At water depth of 12.17 ft:

- WSE = 684.67 ft (NGVD29)
- A (X-sec Area) = 807.28 ft²
- P (Wetted Perimeter) = 92.33 ft
- R (Hydraulic Radius) = 8.743 ft
- Q (Discharge) = (1.486 A R^{2/3}S^{1/2})/n = **7200 ft³/s**

Zone A Workshop

How to determine Base Flood Elevation (BFE)





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